



# Geological investigation of a proposed 4.5 ML reservoir at Clarendon Vale

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## INTRODUCTION

A reconnaissance geological investigation for a proposed 4.5 ML water storage reservoir at Clarendon Vale sought to provide the Clarence Council with a geological appraisal of the site, with particular emphasis on the likely excavation conditions that could be encountered.

The reservoir is to be built on Cuthbertson's property on the western slopes of Stanfields Hill (537420 mE, 5250400 mN) (fig. 1). The site has a moderate slope and the construction of the reservoir at about the 94 m contour level will entail a substantial cut of up to eight metres below the natural surface.

## GEOLOGY

The Hobart 1:50 000 scale geological map sheet (Leaman, 1972) indicates that the reservoir will be located close to the western margin of a narrow north-trending dolerite body which has intruded Triassic-Permian age sedimentary rocks.

The site visit confirmed this to be the case. The reservoir is underlain by dolerite with sandstone-siltstone sedimentary rocks occurring further downslope.

The surface geology at the site consists of a high plasticity clay with scattered dolerite float (boulders) across the slope. There was no outcrop noted on site.

## SEISMIC REFRACTION SURVEY

A four-spread seismic refraction survey was carried out to ascertain the nature of the subsurface materials with respect to the type of excavation conditions that could be expected. A twelve-channel seismograph, with 2.0 and 3.0 m geophone spacings, was used. All spreads were fired from both ends using gelignite and electric detonators. The spread locations are shown on Figure 2 and the results of the survey are summarised in Table 1.

The velocity plots are typically asymmetrical with some stepped segments indicating that variable conditions can be anticipated across the site. The  $V_2$  velocity layer, whilst observed in all spreads, was recorded from one end only in three of the four spreads. This suggests that the material representing this velocity segment lenses in and out over the site.

Materials represented by the  $V_3$  velocity layer are likely to vary considerably from a highly weathered low strength rock (1350 m/s) through to hard, high strength slightly weathered dolerite (2100 m/s).

The layer depth figures expressed in Table 1 should not be regarded as absolute values but rather represent average depths of the various interpretation methods used.

## EXCAVATION CONDITIONS

The rippability guide chart (fig. 3) relates the excavation capability of heavy machinery (D9 or similar) to seismic velocities over a range of rock types. The chart indicates that dolerite is rippable for velocities up to 1800 m/s. Between 1800 and 2500 m/s, ripping is considered marginal and ultimately depends largely on the orientation and intensity of fracturing of the rock mass.

The results of the survey indicate that whilst conditions are likely to vary across the site, the majority of the material to be excavated should be rippable. The use of a rock breaker would be anticipated for the materials represented by the upper end of the  $V_3$  velocity layer (spread 1) where ripping may be marginal. This will depend on the nature of the jointing. It is possible that minor isolated blasting may be necessary.

The high velocity  $V_4$  layer only becomes applicable in spread 1 where excavation depths of around 8.0 m are anticipated. These materials would certainly require extensive blasting but are only anticipated to occur at about the base excavation level and below.

## SUMMARY

The site is covered with 1.0–2.0 m of clay soil and underlain by an irregularly weathered dolerite body.

The seismic refraction survey results indicate that the majority of the materials are rippable to the required depth of excavation, although some zones are considered marginal and workability will ultimately be a product of the joint geometry and weathering characteristics of the rock mass. It is envisaged there will be little need for explosives.

Major stability problems associated with the excavated face are not anticipated, although minor isolated rock falls and/or the dislodgement of individual blocks can be expected from time to time as a result of natural degradation processes.

## REFERENCE

LEAMAN, D. E. 1972. *Geological Atlas 1:50 000 scale series. Sheet 82 (8312S). Hobart.* Department of Mines, Tasmania.

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**Figure 1**  
*Location of proposed reservoir*

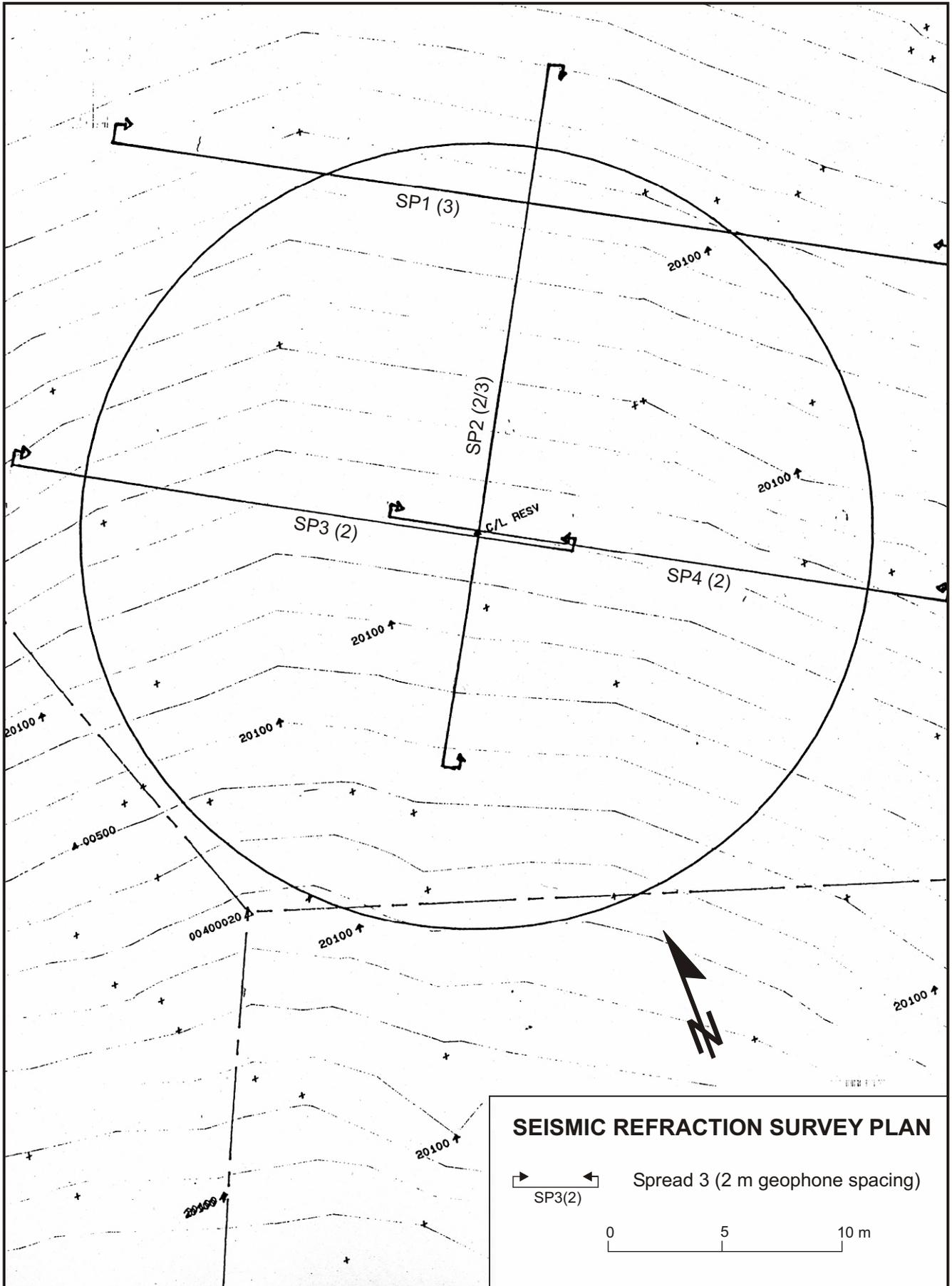


Figure 2

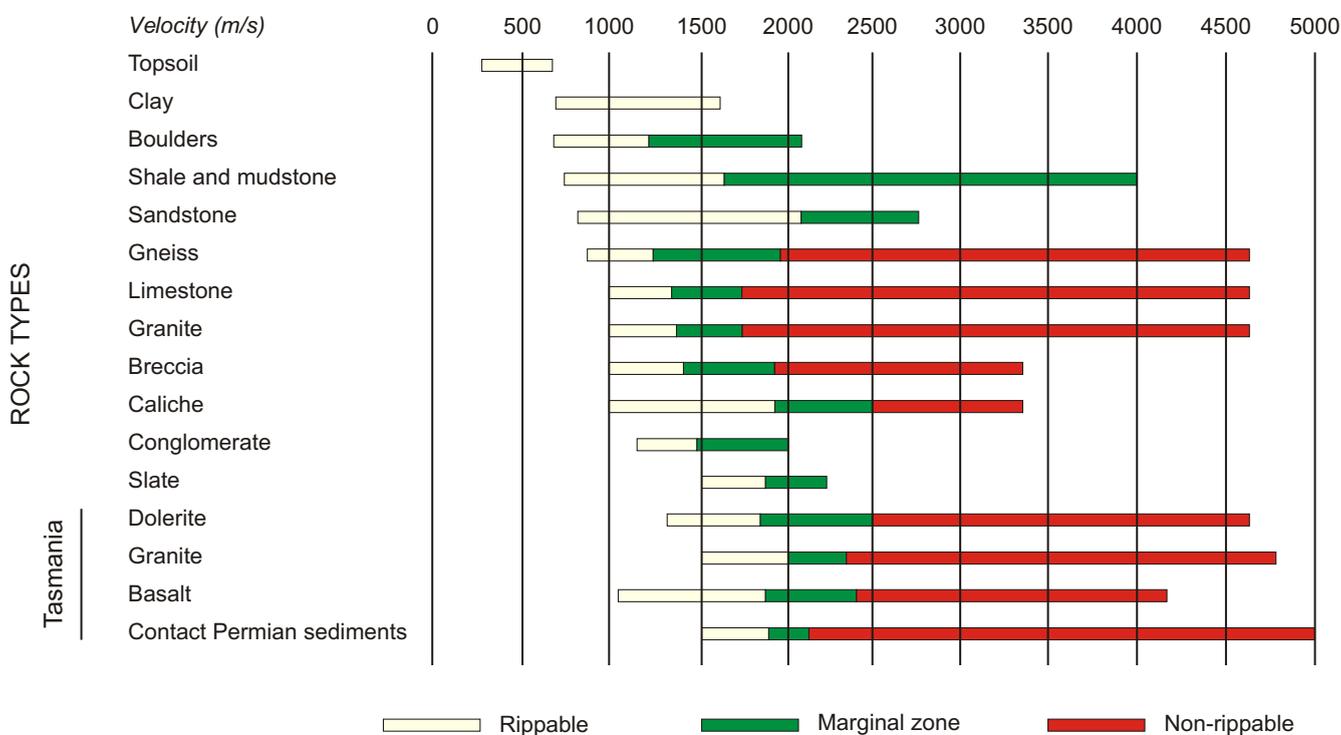
**TABLE 1**  
*Seismic refraction survey, Clarendon Vale reservoir site*

Spread No.	Rock type	Velocity (m/s)	Layer depth (m)	Thickness (m)	Geological interpretation
1	Dolerite	V <sub>1</sub> : 375–430	1.0–1.4	1.0–1.4	Topsoil — surface clay layer
		V <sub>2</sub> : 1090*	3.2	2.2	Residual clay and boulders and/or EW–HW
		V <sub>3</sub> : 1850–2100	8.0–11.0	6.6–7.8	MW–SW rock; joints open–tight
		V <sub>4</sub> : 4500			FR rock; massive–tightly jointed
2	Dolerite	V <sub>1</sub> : 350–500	1.1–2.2	1.1–2.3	Topsoil — surface clay layer
		V <sub>2</sub> : 800*	3.2	2.1	Residual clay and boulders
		V <sub>3</sub> : 1650–1700	10–12 <sup>†</sup>	7–10 <sup>†</sup>	MW–SW rock; joints generally open
3	Dolerite	V <sub>1</sub> : 350–545	1.2–1.8	1.2–1.8	Topsoil — surface clay layer
		V <sub>2</sub> : 890*	3.2	2.0	Residual clay and boulders
		V <sub>3</sub> : 1350–1600	5.2(+)	2.0(+)	HW–MW rock; joints open some clay fill
		V <sub>4</sub> : 2650*			SW–FR rock; joints tight
4	Dolerite	V <sub>1</sub> : 350–375	1.0–1.1	1.0–1.1	Topsoil — surface clay layer
		V <sub>2</sub> : 670–890	2.9–4.3	1.9–3.2	Residual clay and boulders
		V <sub>3</sub> : 1850–2000	7.3(+)	4.4(+)	SW rock; joints open–tight
		V <sub>4</sub> : 4000			FR rock; massive to tightly jointed

<sup>†</sup> : minimum layer depth assuming V<sub>4</sub> = 3000 m/s

\* : Velocity recorded from one end only

FR = fresh, SW = slightly weathered, MW = moderately weathered, HW = highly weathered, EW = extremely weathered



**Figure 3**  
*Guide to rippability (adapted from Soil Test Inc.)*